

Dupuitova rovnice

hydraulická vodivost	k	1.00E-07 m/s	$Q = \frac{2\pi k(H_2^2 - H_1^2)}{\ln(r_2/r_1)}$
snížení na hladinu	H1	50 m	
původní výška vodního sloupce	H2	100 m	
strana čtvercového půdorysu šachty	r1	5 m	
převod nap oloměr kruhového půdorysu	r1'	2.820948 m	Sichardtová
poloměr deprese	r2	47.43 m	r2=3000*(c)
přítok do šachty	Q	1.67E-03 m ³ /s 1.7 l/s	

$$\frac{H_1^2}{\gamma_1})$$

3 rovnice
 $\delta H \rangle_{\text{odm}}(k)$

Goodman et al. (1965)

hydraulické k	5.00E-07 m/s
h	100 m
d	5 m
r'	2.820947918 m
	7.37E-05 m ³ /s/m
Q	0.0737 l/s/m
délka štoly	100 m
	7.3725 l/s

Literature

Goodman e

Zhang and

Lei (1999);
Wagner (

El Tani (19

Karlsrud (2

Moon and]

¹ R_x is the hor
drawdown fror

Su K, Zhou '

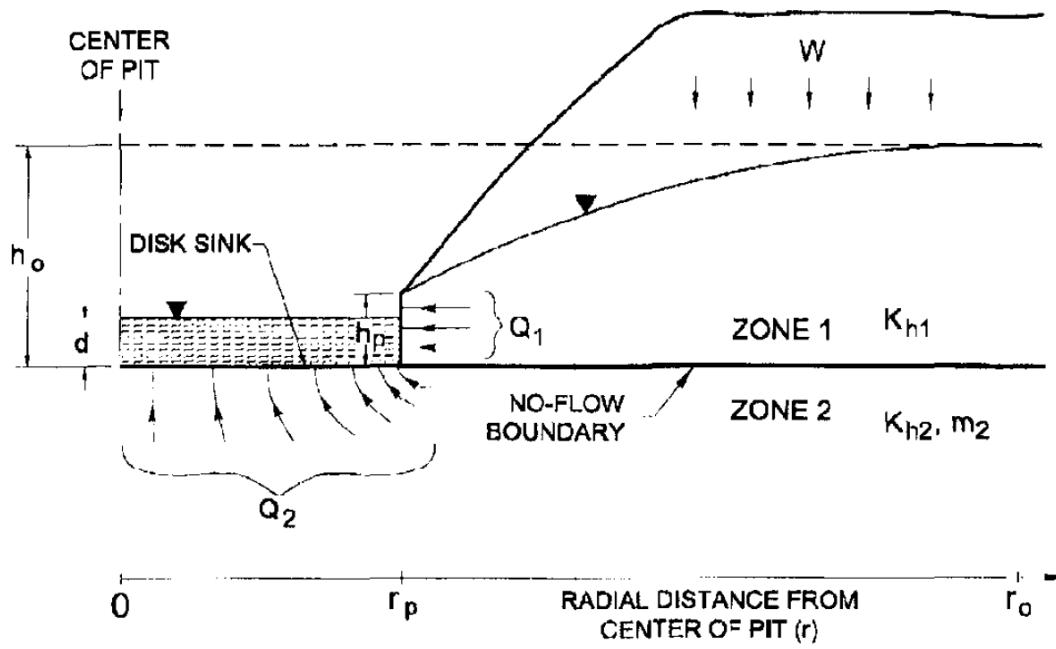
Approximate Solutions of the Groundwater Inflow

	Formula	Description
et al. (1965)	$Q_{Go} = 2\pi k \frac{h}{\ln \frac{2h}{r}}$	Initial water level, deep tunnels, homogeneous, isotropic and semi-infinite aquifer
Franklin (1993)	$Q_{ZF} = 2\pi k \frac{h}{\ln \sqrt{1 + \frac{4h^2}{r^2}}}$	Initial water level, varying hydraulic conductivity of medium in jointed rock deep tunnels
Kolymbas and (2007)	$Q_{LK} = 2\pi k \frac{h}{\ln \left(\frac{h}{r} + \sqrt{\frac{h^2}{r^2} - 1} \right)}$	Initial water level, for both deep and shallow tunnels
(99)	$Q_{EI} = 2\pi kh \frac{1-3\left(\frac{r}{2h}\right)^2}{\left[1-\left(\frac{r}{2h}\right)^2\right] \ln \frac{2h}{r} - \left(\frac{r}{2h}\right)^2}$	Initial water level, tunnels of circular, elliptical or square cross-sections, non-homogeneous aquifer
(2001)	$Q_{Ka} = 2\pi k \frac{h}{\ln \left(\frac{2h}{r} - 1 \right)}$	Initial water level, homogeneous, isotropic and semi-infinite aquifer
Fernandez (2010) ¹	$Q_{MF1} = \frac{k(2R_y h - h^2)}{R_x - r}$ (shallow tunnel) $Q_{MF2} = 2\pi k \frac{h}{\ln \frac{2h}{r}}$ (deep tunnel)	Lowered water level, using permeability reduction of medium, for both deep and shallow tunnels

horizontal influence distance of groundwater level drawdown from the center of tunnel, and R_y is the vertical influence distance of groundwater level in the initial groundwater level.

Y, Wu H, Shi C, Zhou L. An Analytical Method for Groundwater Inflow into a Drained Circular Tunnel. Groundwater, 2010, 42(1), 10-16.

d Water. 2017 Sep;55(5):712-721. doi: 10.1111/gwat.12513. Epub 2017 Mar 22. PMID: 28329431.



Marinelli a Nicolli (2000)

r_p	200	m	
h_o	10	m	25
h_p	5	m	5.40E-05
d	3	m	1156700.25
W	2.70E-09	m/s	1.68222358
k_{h1}	5.00E-05	m/s	558350.125
k_{h2}	5.00E-05	m/s	9.99E+01
k_{v2}	1.00E-05	m/s	
m_2	2.24	m/s	
r_o	1075.5	m ==>>> h_o	10.00
Q_1	9.47E-03 m ³ /s		9.467 l/s
Q_2	1.25E-01 m ³ /s		125.220 l/s
celkem	1.35E-01 m ³ /s		
celkem	134.69 l/s		

Marinelli, Fred, and Niccoli, W.L., 2000, Simple analytical equations for estimating ground water inflow to a river

$$h_o = \sqrt{h_p^2 + \frac{W}{K_{h1}} \left(r_o^2 \ln \left(\frac{r_o}{r_p} \right) - \left(\frac{r_o^2 - r_p^2}{2} \right) \right)}$$

$$Q_1 = W\pi \left(r_o^2 - r_p^2 \right)$$

$$Q_2 = 4r_p \left(\frac{K_{h2}}{m_2} \right) (h_o - d)$$

$$m_2 = \sqrt{\frac{K_{h2}}{K_{v2}}}$$

line pit: *Groundwater*, v. 38, no. 2, p. 311-314.